

## SCHEDULE OF SPECIAL INSPECTIONS

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION (TABLE 1704.4)		CONTINUOUS	PERIODIC
1. Inspection of reinforcing steel and placement.		-----	X
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5b.		NA	NA
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased or where strength design is used.		X	-----
4. Inspection of anchors installed in hardened concrete		NA	NA
5. Verifying use of required design mix.		-----	X
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.		-----	X
7. Inspection of concrete placement for proper application techniques.		-----	X
8. Inspection for maintenance of specified curing temperature and techniques.		NA	NA
9. Inspection of prestressed concrete:		NA	NA
a. Application of prestressing forces		NA	NA
b. Grouting of bonded prestressing tendons in the seismic force-resisting system.		NA	NA
10. Erection of precast concrete members.		NA	NA
11. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.		NA	NA
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.		NA	NA
REQUIRED VERIFICATION AND INSPECTION OF SOILS (TABLE 1704.7)		CONTINUOUS	PERIODIC
1. Verify materials below footings are adequate to achieve the desired bearing capacity.		-----	X
2. Verify excavations are extended to proper depth and have reached proper material.		-----	X
3. Perform classification and testing of controlled fill materials.		-----	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of controlled fill.		X	-----
5. Prior to placement of controlled fill, observe subgrade and verify site has been prepared properly		-----	X
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION (TABLE 1704.3)		CONTINUOUS	PERIODIC
1. Material verification of high-strength bolts, nuts and washers:			
a. Identification markings to conform to ASTM standards specified in the approved construction documents.		-----	X
b. Manufacturer's certificate of compliance required.		-----	X
2. Inspection of high-strength bolting:			
a. Bearing-type connections, snug tight joints.		-----	X
b. Slip critical connections.		NA	NA
3. Material verification of structural steel & Cold-formed steel deck:			
a. For Structural Steel, Identify markings to conform to AISC 360		-----	X
b. For other steel, identify markings to conform to ASTM standards specified in the approved construction documents.		-----	X
c. Manufacturer's certified test reports.		-----	X
4. Material verification of weld filler materials:			
a. Identification markings to conform to AWS specifications in the approved construction documents.		-----	X
b. Manufacturer's certificate of compliance required.		-----	X
5. Inspection of Welding:*			
a. Structural Steel and Cold-formed Steel Deck:			
1) Complete and partial penetration groove welds.		NA	NA
2) Multipass fillet welds.		NA	NA
3) Single-pass fillet welds > 3/8"		NA	NA
4) Plug and slot welds		NA	NA
5) Single-pass fillet welds ≤ 3/8"		NA	X
6) Floor and roof deck welds.		NA	NA
b. Reinforcing steel:			
1) Verification of weldability of reinforcing steel other than ASTM A 706		NA	NA
2) Reinforcing steel-resisting flexural and axial forces in intermediate and special moment frames, boundary elements of special reinforced concrete shear wall and shear reinforcement.		NA	NA
3) Shear reinforcement.		NA	NA
4) Other reinforcement.		NA	NA
6. Inspection of steel frame joint details for compliance with approved construction documents:			
a. Details such as bracing and stiffening.		-----	X
b. Member Locations.		-----	X
c. Application of joint details at each connection.		-----	X

\* Inspections include shop as well as field welding. Special inspections required by this code are not required where work is done on the premises of a fabricator registered and approved to perform such work without special inspection.

## STRUCTURAL NOTES (REFER TO PROJECT MANUAL FOR ADDITIONAL INFORMATION)

### 1. FOOTINGS & FOUNDATION EXCAVATION:

- A. A GEOTECHNICAL ANALYSIS HAS BEEN PERFORMED ON THIS SITE. SEE PROJECT MANUAL FOR GEOTECHNICAL INFORMATION. APPROPRIATE RECOMMENDATIONS STATED IN THE GEOTECHNICAL REPORT ISSUED BY ATC ASSOCIATES, INC. DATED 7-17-12 SHALL BE FOLLOWED.
- B. THESE FOUNDATIONS HAVE BEEN DESIGNED FOR A SOIL BEARING OF 1500 psf FOR CONTINUOUS AND 1500 psf FOR ISOLATED FOOTINGS.
- C. FOUNDATIONS AND SLAB SHOULD BEAR ON COMPETENT NATURAL SOILS OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL. SEE GEOTECHNICAL REPORT FOR SPECIFIC REQUIREMENTS REGARDING EXCAVATION AND PREPARATION OF SUBGRADE. A GEOTECHNICAL ENGINEER SHOULD BE PRESENT TO DIRECT THE REMOVAL OF UNSUITABLE SOILS AND TO DETERMINE THE ADEQUACY OF THE BEARING SURFACE PRIOR TO PLACEMENT OF REINFORCEMENT AND CONCRETE.
- D. FOOTING WIDTHS TO BE AS SHOWN ON PLANS AND DETAILS. BOTTOM OF FOOTING IS TO BE EXCAVATED SQUARE AND TRUE.
- E. NO FOOTING TRENCH SHALL BE OPENED WITHOUT HAVING REINFORCING AND CONCRETE READY TO BE PLACED WITHIN THAT WORKING DAY. ANY VARIATION FROM THIS PROCEDURE SHALL ONLY BE UPON THE APPROVAL OF THE PROJECT ARCHITECT.
- F. ALL STOP FOOTINGS SHALL BE CENTERED UNDER WALLS BEING SUPPORTED AND ALL ISOLATED FOOTINGS SHALL BE CENTERED UNDER COLUMNS, UNLESS NOTED OTHERWISE.
- G. MINIMUM EXTERIOR FOOTING DEPTH BELOW FINISH GRADE SHALL BE AS NOTED ON FOUNDATION PLAN SHEET S2.
- H. IN THE EVENT THAT ORGANIC SOIL OR UNCOMPACTED FILL IS FOUND BELOW FOOTINGS OR FLOOR SLABS, IT SHALL BE REMOVED AND REPLACED WITH SELECT FILL, COMPACTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
- I. STRUCTURAL FILL SHOULD BE PLACED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT. ADEQUATE DENSITY AND MOISTURE CONTENT TESTS SHOULD BE PERFORMED TO INSURE COMPLIANCE WITH PROJECT SPECIFICATIONS. SUBGRADE INSPECTION AND FILL TESTING UNDER CONTROLLED CONDITIONS IS CONSIDERED ESSENTIAL. IF THE FOOTINGS ARE TO BE FOUNDED IN FILL, A TESTING FREQUENCY OF AT LEAST ONE FIELD DENSITY TEST FOR EACH 2500 SQUARE FEET OF LIFT, BUT NOT LESS THAN 3 TESTS PER LIFT IS RECOMMENDED WITHIN THE BUILDING AREAS.

### 2. CONCRETE:

- A. ALL READY MIX CONCRETE SHALL BE 4000 psi FOR ALL CONCRETE PLACEMENT. DO NOT ADD WATER TO THE MIX DESIGN AFTER DELIVERY TO THE PROJECT SITE.
- B. EXPOSED EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED (TOTAL AIR CONTENT = 5%). INTERIOR CONCRETE SHALL NOT BE AIR-ENTRAINED.
- C. UNLESS NOTED OTHERWISE, CONCRETE COVER OVER STEEL REINFORCEMENT SHALL CONFORM TO THE MINIMUM REQUIREMENT BY ACI 318.
- D. REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM TO ACI 318 AND ACI 315, EXCEPT WHERE OTHERWISE INDICATED.
- E. HOT OR COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305-89 AND ACI 306-1-90, RESPECTIVELY.
- F. ANY CONCRETE PLACED BY MEANS OF PUMPING SHALL BE DONE IN ACCORDANCE WITH ACI 304.2R (82).
- G. CEMENT SHALL CONFORM TO A.S.T.M. C-150 TYPE I.
- H. AGGREGATES SHALL CONFORM TO A.S.T.M. C-33 FOR NORMAL WEIGHT CONCRETE & A.S.T.M. C-330 FOR LIGHTWEIGHT CONCRETE.
- I. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH A.S.T.M. C-94.
- J. ADMIXTURES MAY BE USED WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. ADMIXTURES USED TO INCREASE THE WORKABILITY OF THE CONCRETE SHALL NOT BE CONSIDERED TO REDUCE THE CEMENT CONTENT. NO CALCIUM CHLORIDE ADMIXTURES ALLOWED.

### 3. SLABS ON GRADE:

- A. FLOOR SLABS ARE TO BE PLACED AND FINISHED IN ACCORDANCE WITH ACI 302 (SEE PROJECT MANUAL FOR ADDITIONAL INFORMATION).
- B. THICKNESS TOLERANCE FOR ALL SLABS IS TO BE PER ACI 117 AND IS TO BE NO MORE THAN +3/8" (THICKER) AND NO MORE THAN -1/4" (THINNER) FROM THE DESIGN THICKNESS.

### 4. REINFORCING:

- A. REINFORCING BARS SHALL BE BILLET STEEL, ASTM A 615, GRADE 60. PROVIDE CONTINUOUS BENT BARS AT FOOTING STEPS AND 90 DEGREE BENT TIES AT CORNERS. UNLESS OTHERWISE NOTED, LAP SPICES OR EMBEDMENT LENGTHS SHALL CONFORM TO CLASS B SPICE (SEE SPICE TABLE). ADJACENT BAR SPICES IN WALLS AND FOOTINGS TO BE ALTERNATED. ALL FOOTINGS SHALL REQUIRED HOOKED REINFORCING PROJECTED INTO WALLS, PLASTERS OR COLUMNS. THE SIZE AND SPACING OF DOWELS ARE TO MATCH VERTICAL REINFORCING.
- B. WELDED WIRE FABRIC (WVF) SHALL CONFORM TO THE CURRENT ASTM SPECIFICATION FOR COLD DRAWN STEEL REINFORCEMENT WIRE. LAP END AND EDGES MINIMUM 6".
- C. REINFORCING DETAILING, BENDING, AND PLACING SHALL CONFORM TO ACI 315.
- D. MINIMUM CONCRETE COVER: THE MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL AND FACE OF CONCRETE SHALL BE MAINTAINED UNLESS NOTED OTHERWISE:  
SLABS ON EARTH.....CENTER OF SLAB  
CONCRETE BELOW GRADE, FORMED.....2"  
CONCRETE BELOW GRADE, UNFORMED AND  
POURED AGAINST EARTH.....3"

### 5. LUMBER:

- A. TREATED LUMBER:  
1. IN LOCATIONS WHERE TREATED LUMBER IS SHOWN ON DRAWINGS, THE APPROVED PRESSURE TREATED WOODS ARE ACQ-D(CARBONATE) OR CA-B TREATED WOODS WITHOUT AMMONIA CARRIERS. THE CHEMICAL RETENTION LEVELS ARE TO BE NO GREATER THAN 0.4 PCF FOR ACQ-2, 0.21 PCF FOR CA-B. ALL METAL CONNECTORS ARE TO HAVE A GALVANIZED COATING OF NO LESS THAN 1.85 OUNCES OF ZINC PER SQUARE FOOT PER ASTM A153. SCREWS, NAILS AND OTHER FASTENERS ARE TO BE GALVANIZED PER ASTM A153. WHERE TREATED LUMBER IS SHOWN IN EXTERIOR INSTALLATIONS WITH NO ROOF COVERINGS TO PREVENT DIRECT EXPOSURE TO RAIN, USE HOT DIP GALVANIZED CONNECTORS PER ASTM A123.

### 6. LIGHT GAUGE STEEL: SHALL CONFORM TO AISI (LATEST EDITION) AND THE FOLLOWING:

- A. ALL LIGHT GAUGE METAL STUDS, JOISTS AND HEADERS ARE TO MEET OR EXCEED INDUSTRY STANDARDS AS SET FORTH BY THE STEEL STUDS MANUFACTURER'S ASSOCIATION (SSMA).
- B. LIGHT GAUGE STEEL MEMBER DESIGNATIONS SHOWN ON THE CONSTRUCTION DOCUMENTS ARE SSMA STANDARD DESIGNATIONS.
- C. ALL LIGHT GAUGE STEEL WALL SHALL BE LATERALLY BRIDGED USING 1/2" COLD FORM CHANNELS SPACED AT 48" O.C. MAXIMUM VERTICALLY. BRIDGING CHANNEL IS TO BE POSITIVELY CONNECTED AT EACH END BY AN APPROVED METHOD.
- D. BOTTOM TRACK FASTENERS TO BE SPACED AT 48" O.C. MAXIMUM AND WITH 6" OF DOOR / WINDOW OPENINGS AND ENDS OF WALLS.
- E. ALL LOAD BEARING STUDS TO BE SEATED SQUARELY INTO TOP AND BOTTOM WALL TRACKS WITH NO MORE THAN A 1/8" GAP.
- F. THE DESIGN OF SLIP TRACKS SHALL CONFORM TO THE GUIDELINES ESTABLISHED IN SSMA TECHNICAL NOTE NO. 1 PUBLISHED JANUARY, 2001.

### 7. GENERAL:

- A. CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.
- B. THE STRUCTURAL DESIGN OF THE BUILDING IS BASED UPON THE FULL INTERACTION OF ALL ITS COMPONENT PARTS. WITH NO PROVISION MADE FOR CONDITIONS OCCURRING DURING CONSTRUCTION. THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. THE CONTRACTOR SHALL PROVIDE ADEQUATE BRACING DURING CONSTRUCTION. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED AND INSTALLED BY THE CONTRACTOR. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTIBILITY ANALYSIS AND ERECTION PROCEDURES, INCLUDING DESIGN AND ERECTION OF FALSEWORK, TEMPORARY BRACING, ETC. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.
- C. CORRECTIONS DUE TO UNFORESEEN FIELD CONDITIONS OR DIMENSIONAL DISCREPANCIES ON CONSTRUCTION DOCUMENTS MUST BE BROUGHT TO THE ATTENTION OF THE PROJECT ARCHITECT FOR REVIEW AND AUTHORIZATION PRIOR TO CORRECTIVE MEASURES BEING IMPLEMENTED.
- D. STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
- E. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES.
- F. SUBMIT COMPLETE SEALED CALCULATIONS AND DETAILS FOR ALL WINDOW SYSTEMS AND STORE FRONT SYSTEMS FOR APPROVAL.
- G. ALL SHOP DRAWINGS SHALL BE REVIEWED AND APPROVED BY THE PROJECT ARCHITECT/ENGINEER PRIOR TO SUBMITTING TO THE BUILDING DEPARTMENT FOR REVIEW AND APPROVAL.

## SIMPSON "AT-XP" ADHESIVE SYSTEM INTO CONCRETE (IAPMO UES ER-263)

1. SUBSTITUTIONS FOR SIMPSON "AT-XP" ANCHORING ADHESIVE SHALL BE ONLY UPON THE APPROVAL OF THE PROJECT ENGINEER OF RECORD.
2. DRILL PROPER SIZE HOLE IN FULLY GROUTED MASONRY CELLS OR CONCRETE FOR THE DESIGNATED BAR AS SHOWN ON THE PLANS. CLEAN OUT HOLE WITH OIL-FREE COMPRESSED AIR. COMPLETE PREPARATION WITH USE OF A NYLON BRUSH (DO NOT USE WIRE BRUSH). BLOW OUT DUST OR FRAGMENTS.
3. PRIOR TO INJECTION, DISCHARGE AND DISPOSE OF APPROXIMATELY ONE FLUID OUNCE OF ADHESIVE. ADHESIVE MUST BE UNIFORM IN COLOR. INSERT NOZZLE INTO THE BOTTOM OF THE HOLE AND FILL ONE HALF THE HOLE DEPTH.
4. MARK SIDE OF REINFORCEMENT DOWEL OR ANCHOR PRIOR TO PLACEMENT TO INSURE THE ROD IS PLACED TO THE REQUIRED DEPTH.
5. INSERT SELECTED ROD SLOWLY BY HAND INTO THE BOTTOM OF THE HOLE USING A SLOW TWISTING MOTION.
6. LOAD ONLY AFTER RECOMMENDED CURE TIME (SEE MANUFACTURER'S RECOMMENDATIONS).

## DESIGN CRITERIA IS BASED UPON 2010 OREGON STRUCTURAL SPECIALTY CODE

ROOF DEAD LOAD:	5.5 psf	ROOF LIVE LOADS:	20.0 psf
ROOF COLLATERAL:	2.5 psf	SPRINKLER LOAD:	0.0 psf

$$C_e = 1.0 \quad \text{SNOW LOAD: } P_g = 20 \text{ psf} \\ C_t = 1.0 \quad \text{IMPORTANCE FACTOR} = 1.0 \\ P_f = 25 \text{ psf}$$

### WIND CALCULATION METHOD 2

$$\text{WIND SPEED: } V = 95 \text{ mph} \quad \text{EXPOSURE "B"} \\ \text{CATEGORY} = \text{II} \quad \text{IMPORTANCE FACTOR} = 1.00$$

### MAIN WIND FORCE RESISTING SYSTEM

$$\text{WALL AND ROOFS } q = 13.7 \text{ psf} \\ \text{PARAPETS } q_p = 13.7 \text{ psf}$$

### WIND COMPONENTS & CLADDING

Wind pressures shown below were used by JS Smith Consulting Engineers, P.C. for design of the exterior walls. Components designed by others for use in this project will require wind pressures be derived by that supplier.

WALL AREAS 10 SQUARE FEET OR LESS	= 19.8 psf
WALL INTERIOR ZONES	= 12.4 psf & -13.9 psf
WALL END ZONES	= 12.4 psf & -15.1 psf
PARAPETS AT INTERIOR ZONES	= 12.4 psf & -17.6 psf
PARAPETS AT END ZONES	= 12.4 psf & -17.6 psf
ROOF INTERIOR ZONES	= 5.4 psf & -15.1 psf
ROOF EDGE ZONES	= 5.4 psf & -19.0 psf
ROOF CORNER ZONES	= 5.4 psf & -21.0 psf

$$\text{SEISMIC} = 37.7 \text{ K}$$

$$\text{WIND} = 25.1 \text{ K}$$

$$\text{SEISMIC} = 35.0 \text{ K}$$

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